

MYERS ENGINEERING

Cantilever Retaining Walls



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Project: RKK Spec Residence
4115 78th Avenue SE
Mercer Island, WA

April 3, 2025

2021 INTERNATIONAL BUILDING CODE
100 MPH BASIC WIND, EXPOSURE C, $K_{zt} = 1.61$
RISK CATEGORY II - SOIL SITE CLASS D
SEISMIC DESIGN CATEGORY D (IBC)

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Cantilevered Retaining Wall

Project File: 4115 78th AVE SE 056

LIC# : KW-06015659, Build:20.25.03.24

MYERS ENGINEERING

(c) ENERCALC, LLC 1982-2025

DESCRIPTION: 10ft Stem at Basement w/ Slab

Code Reference:

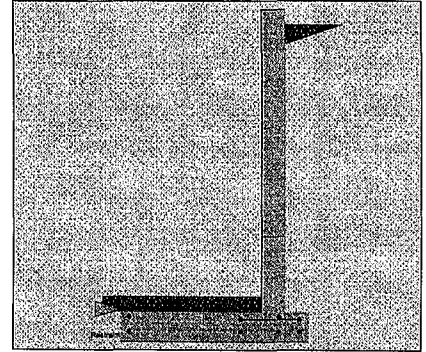
Calculations per IBC 2018, ACI 318-14, TMS 402-16

Criteria

Retained Height	=	9.50 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water table above bottom of footing	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,500.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0 psf
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	150.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	1.3 in

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Cantilevered Retaining Wall Project File: 4115 78th AVE SE.prc

LIC# : KW-06015659, Build:20.25.03.24 MYERS ENGINEERING (c) ENERCALC, LLC 1982-2025

DESCRIPTION: 10ft Stem at Basement w/ Slab

Design Summary	Stem Construction	2nd	Bottom
Wall Stability Ratios	Design Height Above Ftg ft =	Stem OK 2.50	Stem OK 0.00
Overturning = 1.64 OK	Wall Material Above "Ht" =	Concrete	Concrete
Slab Resists All Sliding !	Design Method =	SD	SD
Global Stability = 1.14	Thickness =	8.00	8.00
	Rebar Size =	# 4	# 5
	Rebar Spacing =	10.00	5.00
	Rebar Placed at =	6 in	6 in
Total Bearing Load = 3,718 lbs	Design Data		
...resultant ecc. = 3.46 in	fb/FB + fa/Fa =	0.515	0.465
Eccentricity within middle third	Total Force @ Section		
Soil Pressure @ Toe = 923 psf OK	Service Level lbs =		
Soil Pressure @ Heel = 471 psf OK	Strength Level lbs =	1,372.0	2,527.0
Allowable = 2,500 psf	Moment....Actual		
Soil Pressure Less Than Allowable	Service Level ft-# =		
ACI Factored @ Toe = 996 psf	Strength Level ft-# =	3,182.6	7,983.4
ACI Factored @ Heel = 509 psf	Moment....Allowable ft-# =	6,174.1	17,148.7
Footing Shear @ Toe = 18.6 psi OK	Shear.....Actual		
Footing Shear @ Heel = 19.2 psi OK	Service Level psi =		
Allowable = 75.0 psi	Strength Level psi =	19.1	35.1
Sliding Calcs	Shear....Allowable psi =	75.0	75.0
Lateral Sliding Force = 1,929.4 lbs	Anet (Masonry) in2 =		
	Wall Weight psf =	100.0	100.0
	Rebar Depth 'd' in =	6.00	6.00
	Masonry Data		
	f'm psi =		
	Fs psi =		
	Solid Grouting =		
	Modular Ratio 'n' =		
	Equiv. Solid Thick. =		
	Masonry Block Type =		
	Masonry Design Method =	ASD	
	Concrete Data		
	f'c psi =	2,500.0	2,500.0
	Fy psi =	60,000.0	60,000.0

Vertical component of active lateral soil pressure IS considered in the calculation of soil bearing pressures.

Load Factors

Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

Cantilevered Retaining Wall

Project File: 4115 78th AVE SE 6-5

LIC#: KW-06015659, Build:20.25.03.24

MYERS ENGINEERING

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DESCRIPTION: 10ft Stem at Basement w/ Slab

Concrete Stem Rebar Area Details

2nd Stem	<u>Vertical Reinforcing</u>	<u>Horizontal Reinforcing</u>
As (based on applied moment) :	0.1245 in ² /ft	
(4/3) * As :	0.166 in ² /ft	Min Stem T&S Reinf Area 1.440 in ²
200bd/fy : 200(12)(6)/60000 :	0.24 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :
	=====	<u>One layer of :</u> <u>Two layers of :</u>
Required Area :	0.1728 in ² /ft	#4@ 12.50 in #4@ 25.00 in
Provided Area :	0.24 in ² /ft	#5@ 19.38 in #5@ 38.75 in
Maximum Area :	0.8128 in ² /ft	#6@ 27.50 in #6@ 55.00 in

Bottom Stem	<u>Vertical Reinforcing</u>	<u>Horizontal Reinforcing</u>
As (based on applied moment) :	0.3123 in ² /ft	
(4/3) * As :	0.4164 in ² /ft	Min Stem T&S Reinf Area 0.480 in ²
200bd/fy : 200(12)(6)/60000 :	0.24 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :
	=====	<u>One layer of :</u> <u>Two layers of :</u>
Required Area :	0.3123 in ² /ft	#4@ 12.50 in #4@ 25.00 in
Provided Area :	0.744 in ² /ft	#5@ 19.38 in #5@ 38.75 in
Maximum Area :	0.8128 in ² /ft	#6@ 27.50 in #6@ 55.00 in

Footing Data

Toe Width	=	4.00 ft
Heel Width	=	1.33
Total Footing Width	=	5.33
Footing Thickness	=	12.00 in

f'c =	2,500 psi	Fy =	60,000 psi
Footing Concrete Density	=	150.00 pcf	
Min. As %	=	0.0018	
Cover @ Top	3.00	@ Btm. =	3.00 in

Footing Design Results

		<u>Toe</u>	<u>Heel</u>	
Factored Pressure	=	996	509	psf
Mu' : Upward	=	6,995	117	ft-#
Mu' : Downward	=	1,968	1,226	ft-#
Mu: Design	=	5,027	1,109	ft-#
φ Mn	=	13,810	13,810	ft-#
Actual 1-Way Shear	=	18.65	19.21	psi
Allow 1-Way Shear	=	75.00	75.00	psi
Toe Reinforcing	=	# 5 @ 10.00 in		
Heel Reinforcing	=	# 5 @ 10.00 in		
Key Reinforcing	=	None Spec'd		
Footing Torsion, Tu	=		0.00 ft-lbs	
Footing Allow. Torsion, φ Tn	=		0.00 ft-lbs	

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 9.25 in, #5@ 14.35 in, #6@ 18 in, #7@ 18 in, #8@ 18 in, #9@ 18 in, #10@ 18 in

Heel: #4@ 9.25 in, #5@ 14.35 in, #6@ 18 in, #7@ 18 in, #8@ 18 in, #9@ 18 in, #10@ 18 in

Key: No key defined

Min footing T&S reinf Area	1.38 in ²
Min footing T&S reinf Area per foot	0.26 in ² /ft

<u>If one layer of horizontal bars:</u>	<u>If two layers of horizontal bars:</u>
#4@ 9.26 in	#4@ 18.52 in
#5@ 14.35 in	#5@ 28.70 in
#6@ 20.37 in	#6@ 40.74 in

Cantilevered Retaining Wall

Project File: 4115 78th AVE SE 308

LIC# : KW-06015659, Build:20.25.03.24

MYERS ENGINEERING

(c) ENERCALC, LLC 1982-2025

DESCRIPTION: 10ft Stem at Basement w/ Slab

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	1,929.4	3.50	6,752.8	Soil Over HL (ab. water tbl)	696.3	5.00	3,481.5
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		5.00	3,481.5
Hydrostatic Force				Water Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =	150.0	4.44	665.6
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	220.0	2.00	440.0
				Surcharge Over Toe =			
				Stem Weight(s) =	1,000.0	4.33	4,333.3
				Earth @ Stem Transitions =			
Total	= 1,929.4	O.T.M. =	6,752.8	Footing Weight =	800.0	2.67	2,133.1
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		=	1.64	Total =	2,866.3 lbs	R.M. =	11,053.5
Vertical Loads used for Soil Pressure =			3,717.9 lbs				

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.048 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Cantilevered Retaining Wall

LIC# : KW-06015659, Build:20.25.03.24

MYERS ENGINEERING

(c) ENERCALC, LLC 1982-2025

DESCRIPTION: 10ft Stem at Basement w/ Slab

Rebar Lap & Embedment Lengths Information

Stem Design Segment: 2nd

Stem Design Height: 2.50 ft above top of footing

Lap Splice length for #4 bar specified in this stem design segment (25.4.2.3a) = 18.72 in
Development length for #4 bar specified in this stem design segment = 14.40 in

Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #5 bar specified in this stem design segment (25.4.2.3a) = 23.40 in
Development length for #5 bar specified in this stem design segment = 18.00 in

Hooked embedment length into footing for #5 bar specified in this stem design segment = 6.00 in
As Provided = 0.7440 in²/ft
As Required = 0.3123 in²/ft

Cantilevered Retaining Wall

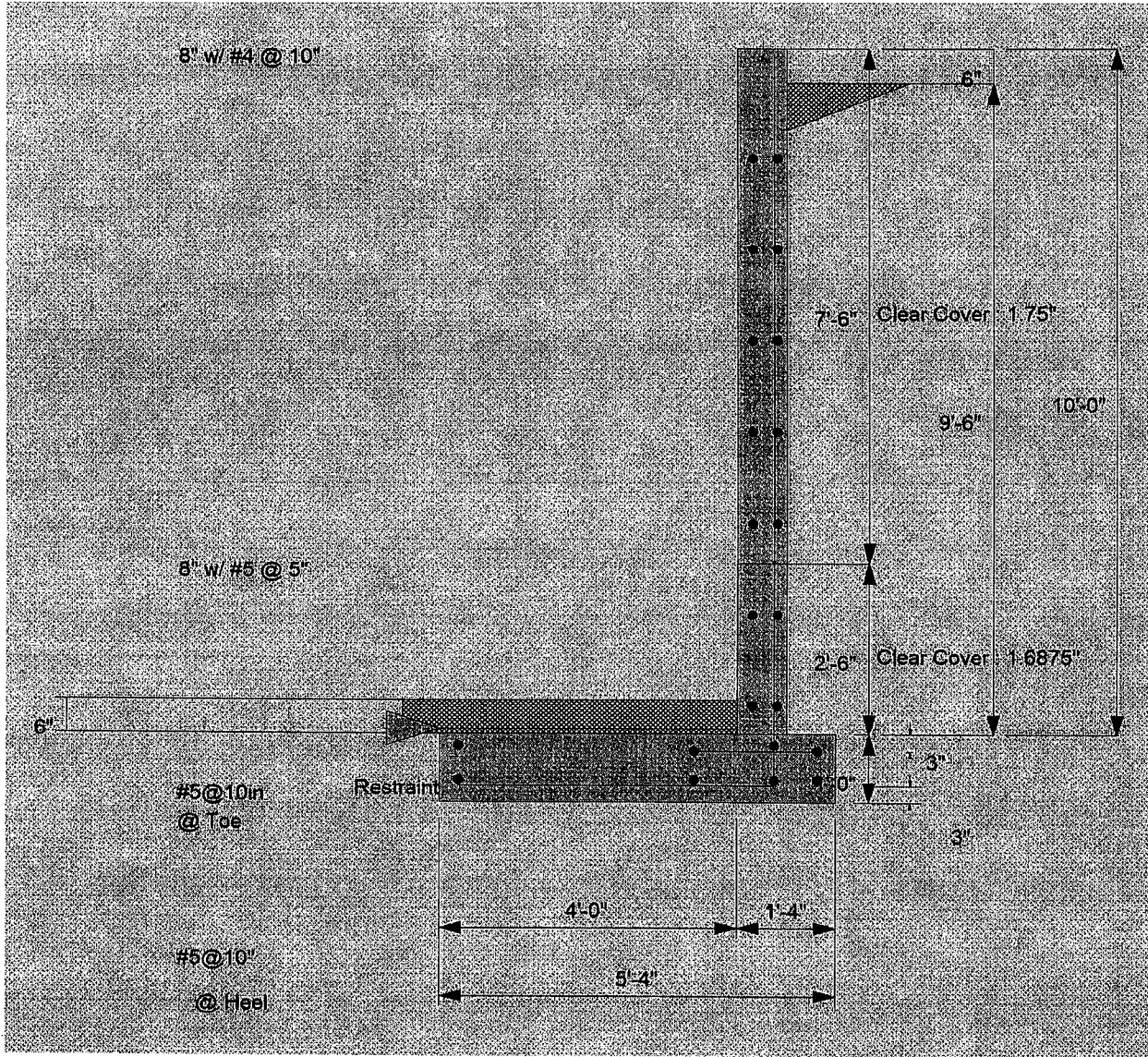
Project File: 4115 78th AVE SE.dwg

LIC# : KW-06015659, Build:20.25.03.24

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DESCRIPTION: 10ft Stem at Basement w/ Slab



Cantilevered Retaining Wall

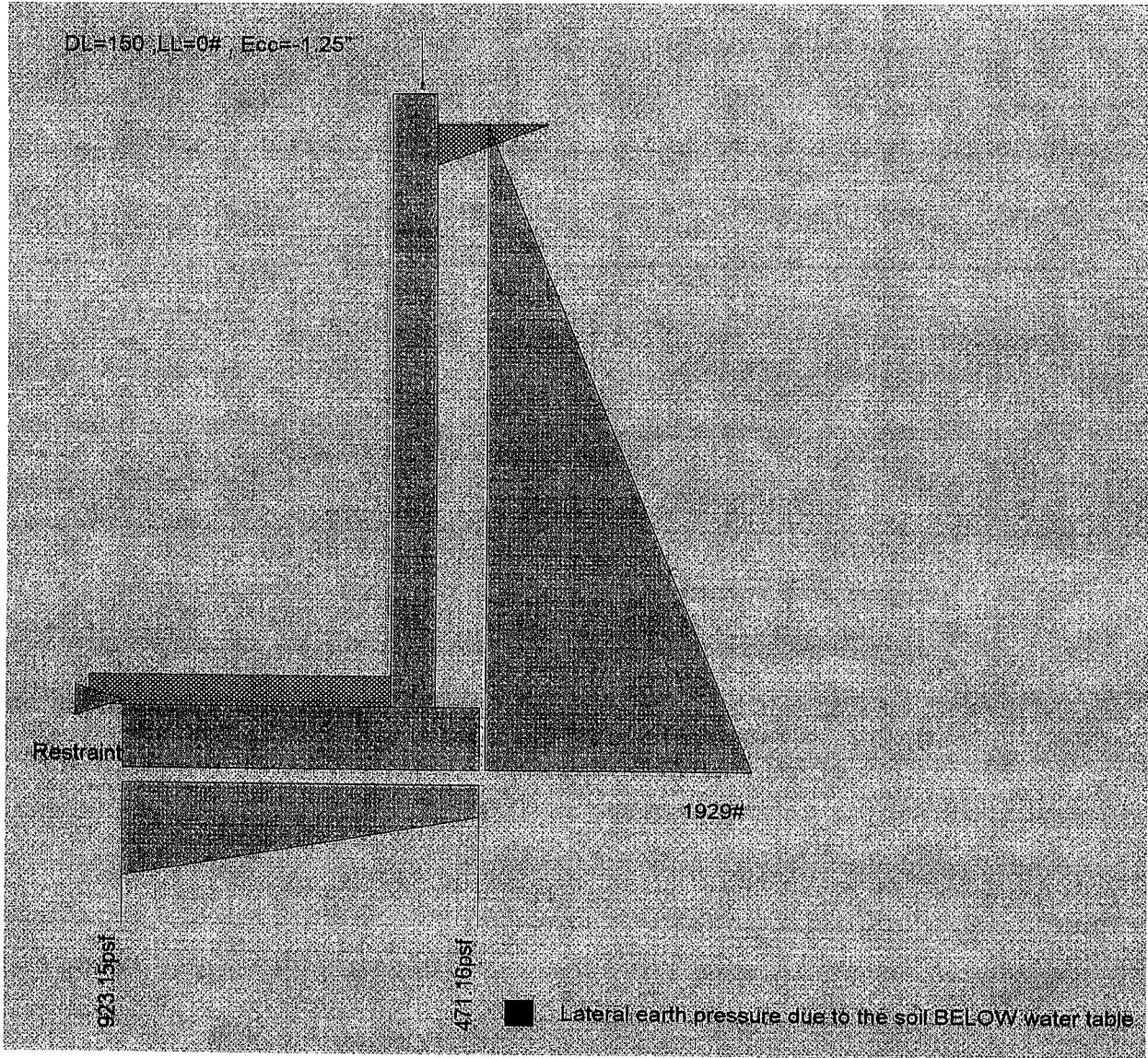
Project File: 4115 78th AVE SE 006

LIC# : KW-06015659, Build:20.25.03.24

MYERS ENGINEERING

(c) ENERCALC, LLC 1982-2025

DESCRIPTION: 10ft Stem at Basement w/ Slab



Cantilevered Retaining Wall

Project File: 4115-78th AVE SE

LIC# : KW-06015659, Build:20.25.03.24

MYERS ENGINEERING

(c) ENERCALC, LLC 1982-2025

DESCRIPTION: 10ft Stem at Basement w/ Slab & Seismic

Code Reference:

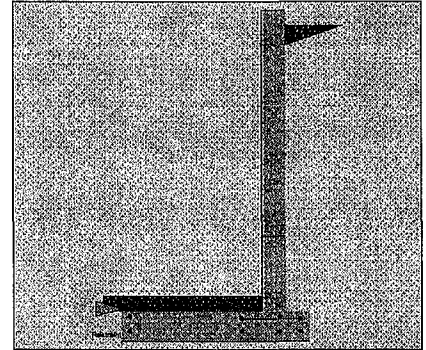
Calculations per IBC 2018, ACI 318-14, TMS 402-16

Criteria

Retained Height	=	9.50 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water table above bottom of footing	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,500.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.0 pcf
Soil Density, Toe	=	110.0 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0 psf
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	150.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	1.3 in

Earth Pressure Seismic Load

Method : Uniform		
Multiplier Used	=	8.000
(Multiplier used on soil density)		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Uniform Seismic Force	=	84.000
Total Seismic Force	=	882.000

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Can/Overlaid Retaining Wall

Project File: 4115 78th AVE SE - 065

LIC#: KW-06015659, Build:20.25.03.24

MYERS ENGINEERING

(c) ENERCALC, LLC 1982-2025

DESCRIPTION: 10ft Stem at Basement w/ Slab & Seismic

Design Summary

Wall Stability Ratios

Overturning	=	1.11 Ratio < 1.5!
Slab Resists All Sliding !		
Global Stability	=	1.14
Total Bearing Load	=	3,718 lbs
...resultant ecc.	=	13.92 in
Eccentricity outside middle third		
Soil Pressure @ Toe	=	1,645 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,500 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	1,776 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	27.8 psi OK
Footing Shear @ Heel	=	22.7 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	2,546.8 lbs
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Vertical component of active lateral soil pressure IS considered in the calculation of soil bearing pressures.

Load Factors

Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

Stem Construction

Design Height Above Ftg

Wall Material Above "Ht"	=	Concrete
Design Method	=	SD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	10.00
Rebar Placed at	=	6 in

Design Data

fb/FB + fa/Fa	=	0.848	0.686
Total Force @ Section			
Service Level	lbs =		
Strength Level	lbs =	1,960.0	3,325.0
Moment....Actual			
Service Level	ft-# =		
Strength Level	ft-# =	5,240.6	11,773.9
Moment....Allowable	ft-# =	6,174.1	17,148.7
Shear.....Actual			
Service Level	psi =		
Strength Level	psi =	27.2	46.2
Shear....Allowable	psi =	75.0	75.0
Anet (Masonry)	in2 =		
Wall Weight	psf =	100.0	100.0
Rebar Depth 'd'	in =	6.00	6.00

Masonry Data

f _m	psi =	
F _s	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	
Masonry Design Method	=	ASD

Concrete Data

f _c	psi =	2,500.0	2,500.0
F _y	psi =	60,000.0	60,000.0

Cantilevered Retaining Wall

Project Title: 4115 78th AVE SE

LIC#: KW-06015659, Build:20.25.03.24

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DESCRIPTION: 10ft Stem at Basement w/ Slab & Seismic

Concrete Stem Rebar Area Details

2nd Stem	<u>Vertical Reinforcing</u>	<u>Horizontal Reinforcing</u>
As (based on applied moment) :	0.205 in2/ft	
(4/3) * As :	0.2733 in2/ft	Min Stem T&S Reinf Area 1.440 in2
200bd/fy : 200(12)(6)/60000 :	0.24 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft
0.0018bh : 0.0018(12)(8) :	0.1728 in2/ft	Horizontal Reinforcing Options :
	=====	<u>One layer of :</u> <u>Two layers of :</u>
Required Area :	0.24 in2/ft	#4@ 12.50 in #4@ 25.00 in
Provided Area :	0.24 in2/ft	#5@ 19.38 in #5@ 38.75 in
Maximum Area :	0.8128 in2/ft	#6@ 27.50 in #6@ 55.00 in

Bottom Stem	<u>Vertical Reinforcing</u>	<u>Horizontal Reinforcing</u>
As (based on applied moment) :	0.4605 in2/ft	
(4/3) * As :	0.614 in2/ft	Min Stem T&S Reinf Area 0.480 in2
200bd/fy : 200(12)(6)/60000 :	0.24 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft
0.0018bh : 0.0018(12)(8) :	0.1728 in2/ft	Horizontal Reinforcing Options :
	=====	<u>One layer of :</u> <u>Two layers of :</u>
Required Area :	0.4605 in2/ft	#4@ 12.50 in #4@ 25.00 in
Provided Area :	0.744 in2/ft	#5@ 19.38 in #5@ 38.75 in
Maximum Area :	0.8128 in2/ft	#6@ 27.50 in #6@ 55.00 in

Footing Data

Toe Width	=	4.00 ft
Heel Width	=	1.33
Total Footing Width	=	5.33
Footing Thickness	=	12.00 in

f'c =	2,500 psi	Fy =	60,000 psi
Footing Concrete Density	=	150.00 pcf	
Min. As %	=	0.0018	
Cover @ Top	3.00	@ Btm.=	3.00 in

Footing Design Results

		<u>Toe</u>	<u>Heel</u>	
Factored Pressure	=	1,776	0	psf
Mu' : Upward	=	10,015	0	ft-#
Mu' : Downward	=	1,968	1,226	ft-#
Mu: Design	=	8,047	1,226	ft-#
φ Mn	=	13,810	13,810	ft-#
Actual 1-Way Shear	=	27.79	22.73	psi
Allow 1-Way Shear	=	75.00	75.00	psi
Toe Reinforcing	=	# 5 @ 10.00 in		
Heel Reinforcing	=	# 5 @ 10.00 in		
Key Reinforcing	=	None Spec'd		
Footing Torsion, Tu	=		0.00 ft-lbs	
Footing Allow. Torsion, φ Tn	=		0.00 ft-lbs	

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 8.23 in, #5@ 12.76 in, #6@ 18 in, #7@ 18 in, #8@ 18 in, #9@ 18 in, #10@ 18 in

Heel: #4@ 9.25 in, #5@ 14.35 in, #6@ 18 in, #7@ 18 in, #8@ 18 in, #9@ 18 in, #10@ 18 in

Key: No key defined

Min footing T&S reinf Area	1.38 in2
Min footing T&S reinf Area per foot	0.26 in2 /ft

If one layer of horizontal bars: If two layers of horizontal bars:

#4@ 9.26 in	#4@ 18.52 in
#5@ 14.35 in	#5@ 28.70 in
#6@ 20.37 in	#6@ 40.74 in

Cantilevered Retaining Wall

Project File: 4115 78th AVE SE

LIC# : KW-06015659, Build:20.25.03.24

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DESCRIPTION: 10ft Stem at Basement w/ Slab & Seismic

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	1,929.4	3.50	6,752.8	Soil Over HL (ab. water tbl)	696.3	5.00	3,481.5
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		5.00	3,481.5
Hydrostatic Force				Water Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =	150.0	4.44	665.6
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	220.0	2.00	440.0
Seismic Earth Load =	617.4	5.25	3,241.4	Surcharge Over Toe =			
				Stem Weight(s) =	1,000.0	4.33	4,333.3
				Earth @ Stem Transitions =			
Total	2,546.8	O.T.M. =	9,994.2	Footing Weight =	800.0	2.67	2,133.1
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		=	1.11	Total =	2,866.3 lbs	R.M. =	11,053.5
Vertical Loads used for Soil Pressure =			3,717.9 lbs				

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

If seismic is included, the OTM and sliding ratios may be 1.1 per section 1807.2.3 of IBC.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.086 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Cantilevered Retaining Wall

Project Title: 4115 78th AVE SE

LIC# : KW-06015659, Build:20.25.03.24

MYERS ENGINEERING

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DESCRIPTION: 10ft Stem at Basement w/ Slab & Seismic

Rebar Lap & Embedment Lengths Information

Stem Design Segment: 2nd

Stem Design Height: 2.50 ft above top of footing

Lap Splice length for #4 bar specified in this stem design segment (25.4.2.3a) = 18.72 in
Development length for #4 bar specified in this stem design segment = 14.40 in

Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #5 bar specified in this stem design segment (25.4.2.3a) = 23.40 in
Development length for #5 bar specified in this stem design segment = 18.00 in
Hooked embedment length into footing for #5 bar specified in this stem design segment = 6.50 in
As Provided = 0.7440 in²/ft
As Required = 0.4605 in²/ft

Cantilevered Retaining Wall

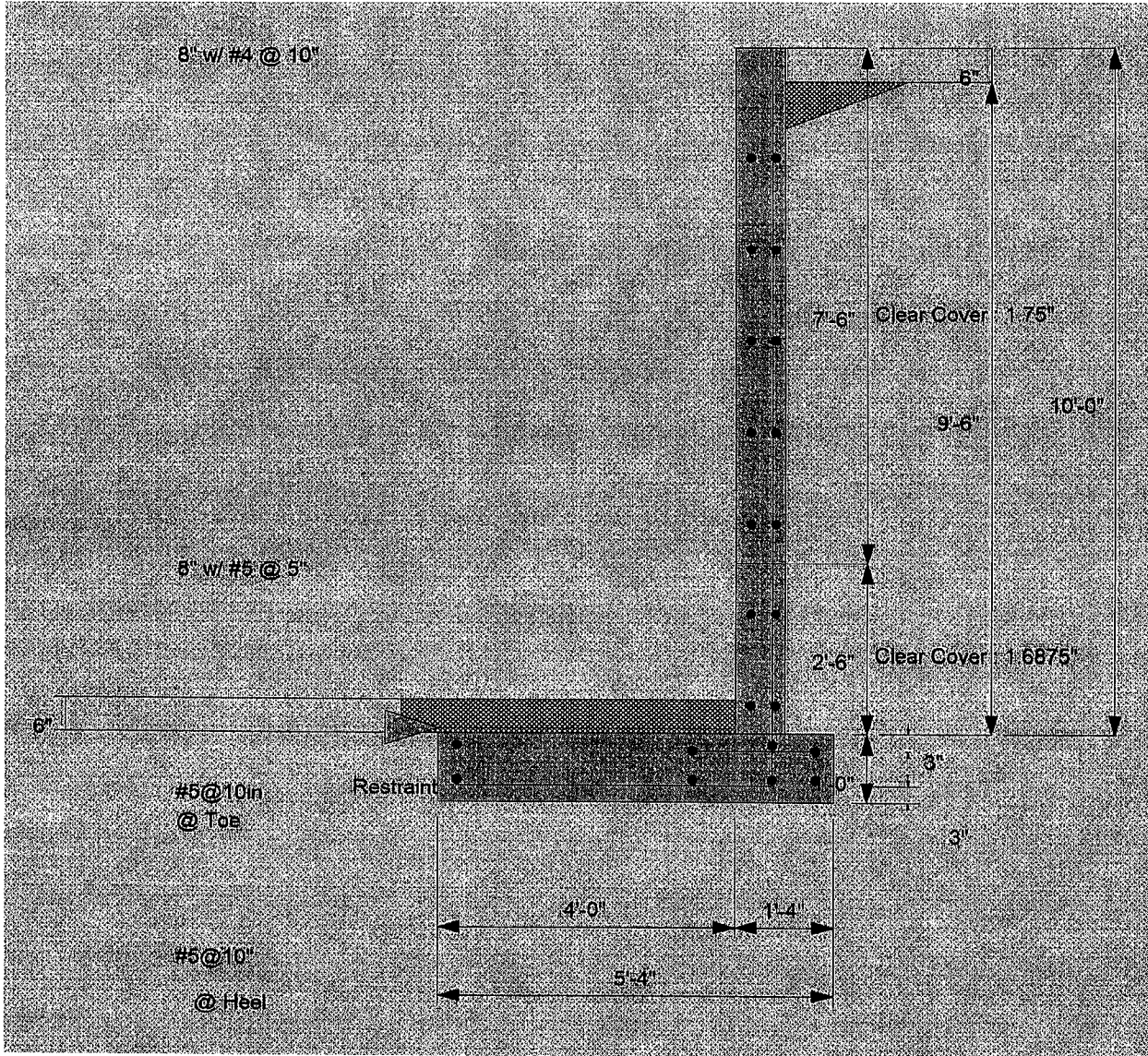
Project #15-2115-78th AVE SE 9-8

LIC# : KW-06015659, Build:20.25.03.24

MYERS ENGINEERING

(c) ENERCALC, LLC 1982-2025

DESCRIPTION: 10ft Stem at Basement w/ Slab & Seismic



Canilevered Retaining Wall

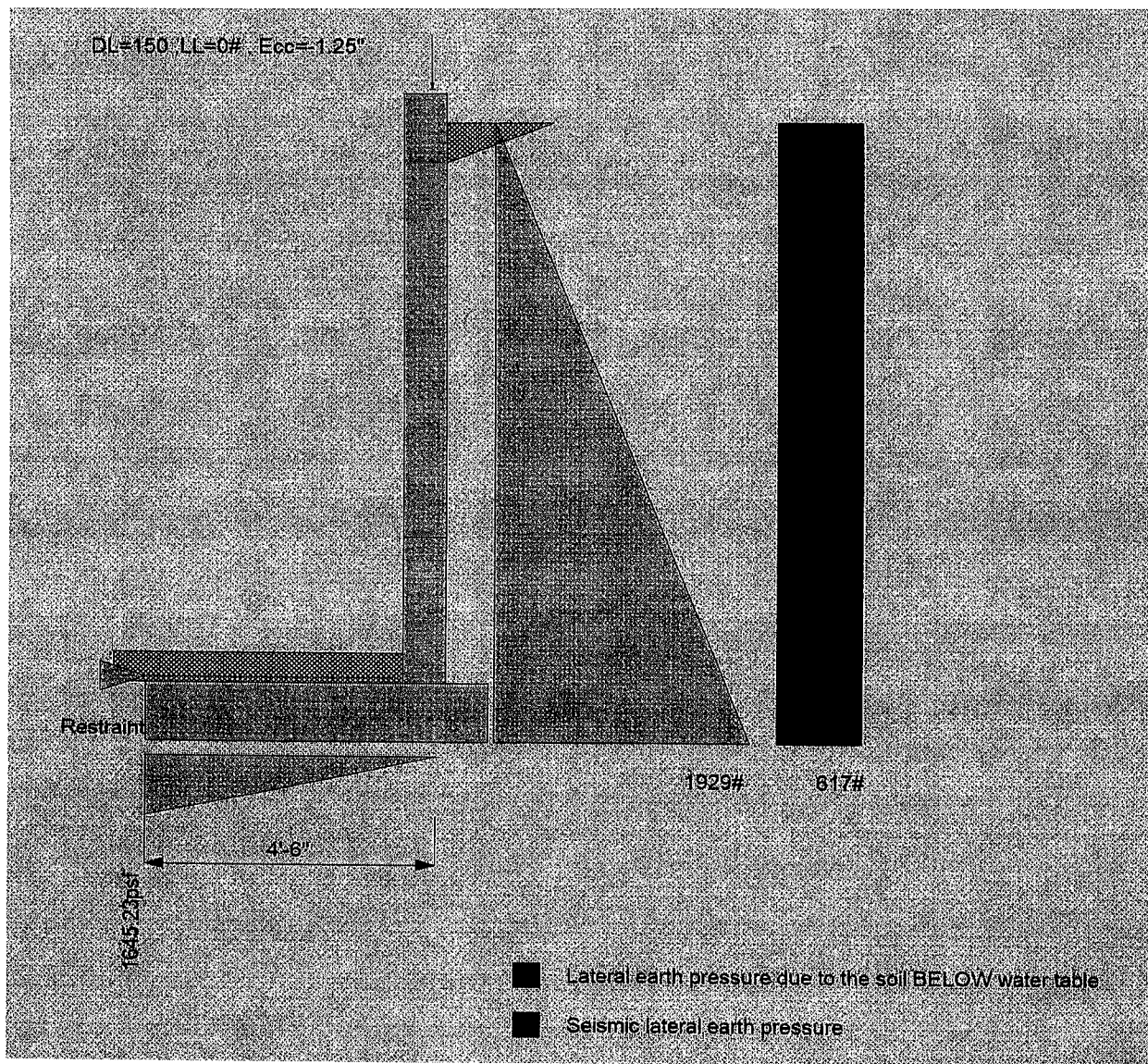
Project File: 4115 78th AVE SE 6-8

LIC# : KW-06015659, Build:20.25.03.24

MYERS ENGINEERING

(c) ENERCALC, LLC 1982-2025

DESCRIPTION: 10ft Stem at Basement w/ Slab & Seismic



Castilevered Retaining Wall

Project File: 4115 78th AVE SE

LIC# : KW-06015659, Build:20.25.03.24

MYERS ENGINEERING

(c) ENERCALC, LLC 1982-2025

DESCRIPTION: 8ft Stem at Basement w/ Slab

Code Reference:

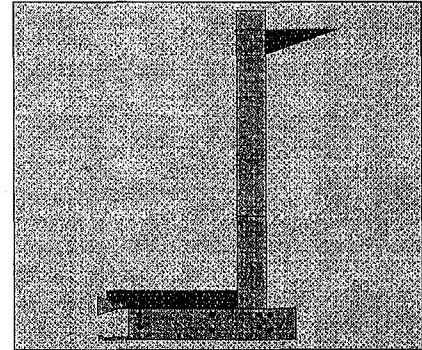
Calculations per IBC 2018, ACI 318-14, TMS 402-16

Criteria

Retained Height	=	7.50 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water table above bottom of footing	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,500.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0 psf
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	150.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	1.3 in

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Canflavored Retaining Wall Project File: 4115 78th AVE SE 10.0

LIC#: KW-06015659, Build:20.25.03.24 MYERS ENGINEERING (c) ENERCALC, LLC 1982-2025

DESCRIPTION: 8ft Stem at Basement w/ Slab

Design Summary	Stem Construction	2nd	Bottom
Wall Stability Ratios	Design Height Above Ftg ft =	Stem OK 2.50	Stem OK 0.00
Overturning = 1.69 OK	Wall Material Above "Ht" =	Concrete	Concrete
Slab Resists All Sliding !	Design Method =	SD	SD
Global Stability = 1.35	Thickness =	8.00	8.00
	Rebar Size =	# 4	# 4
	Rebar Spacing =	12.00	6.00
	Rebar Placed at =	6 in	6 in
Total Bearing Load = 2,653 lbs	Design Data		
...resultant ecc. = 3.09 in	fb/FB + fa/Fa =	0.221	0.393
Eccentricity within middle third	Total Force @ Section		
Soil Pressure @ Toe = 971 psf OK	Service Level lbs =		
Soil Pressure @ Heel = 413 psf OK	Strength Level lbs =	700.0	1,575.0
Allowable = 2,500 psf	Moment....Actual		
Soil Pressure Less Than Allowable	Service Level ft-# =		
ACI Factored @ Toe = 1,084 psf	Strength Level ft-# =	1,147.9	3,918.8
ACI Factored @ Heel = 462 psf	Moment....Allowable ft-# =	5,187.6	9,950.4
Footing Shear @ Toe = 17.3 psi OK	Shear.....Actual		
Footing Shear @ Heel = 42.5 psi OK	Service Level psi =		
Allowable = 75.0 psi	Strength Level psi =	9.7	21.9
Sliding Calcs	Shear....Allowable psi =	75.0	75.0
Lateral Sliding Force = 1,215.3 lbs	Anet (Masonry) in2 =		
	Wall Weight psf =	100.0	100.0
	Rebar Depth 'd' in =	6.00	6.00
	Masonry Data		
	f'm psi =		
	Fs psi =		
	Solid Grouting =		
	Modular Ratio 'n' =		
	Equiv. Solid Thick. =		
	Masonry Block Type =		
	Masonry Design Method = ASD		
	Concrete Data		
	f'c psi =	2,500.0	2,500.0
	Fy psi =	60,000.0	60,000.0

Vertical component of active lateral soil pressure IS considered in the calculation of soil bearing pressures.

Load Factors	
Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

Can'teovered Retaining Wall

LIC#: KW-06015659, Build:20.25.03.24

MYERS ENGINEERING

(c) ENERCALC, LLC 1982-2025

DESCRIPTION: 8ft Stem at Basement w/ Slab

Concrete Stem Rebar Area Details

2nd Stem	<u>Vertical Reinforcing</u>	<u>Horizontal Reinforcing</u>
As (based on applied moment) :	0.0449 in2/ft	
(4/3) * As :	0.0599 in2/ft	Min Stem T&S Reinf Area 1.056 in2
200bd/fy : 200(12)(6)/60000 :	0.24 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft
0.0018bh : 0.0018(12)(8) :	0.1728 in2/ft	Horizontal Reinforcing Options :
	=====	<u>One layer of :</u> <u>Two layers of :</u>
Required Area :	0.1728 in2/ft	#4@ 12.50 in #4@ 25.00 in
Provided Area :	0.2 in2/ft	#5@ 19.38 in #5@ 38.75 in
Maximum Area :	0.8128 in2/ft	#6@ 27.50 in #6@ 55.00 in

Bottom Stem	<u>Vertical Reinforcing</u>	<u>Horizontal Reinforcing</u>
As (based on applied moment) :	0.1533 in2/ft	
(4/3) * As :	0.2044 in2/ft	Min Stem T&S Reinf Area 0.480 in2
200bd/fy : 200(12)(6)/60000 :	0.24 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft
0.0018bh : 0.0018(12)(8) :	0.1728 in2/ft	Horizontal Reinforcing Options :
	=====	<u>One layer of :</u> <u>Two layers of :</u>
Required Area :	0.2044 in2/ft	#4@ 12.50 in #4@ 25.00 in
Provided Area :	0.4 in2/ft	#5@ 19.38 in #5@ 38.75 in
Maximum Area :	0.8128 in2/ft	#6@ 27.50 in #6@ 55.00 in

Footing Data

Toe Width	=	2.50 ft
Heel Width	=	1.33
Total Footing Width	=	3.83
Footing Thickness	=	10.00 in
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	7.00	@ Btm.= 3.00 in

Footing Design Results

		<u>Toe</u>	<u>Heel</u>	
Factored Pressure	=	1,084	462	psf
Mu' : Upward	=	2,965	111	ft-#
Mu' : Downward	=	675	825	ft-#
Mu: Design	=	2,290	714	ft-#
φ Mn	=	6,985	2,665	ft-#
Actual 1-Way Shear	=	17.25	42.47	psi
Allow 1-Way Shear	=	75.00	75.00	psi
Toe Reinforcing	=	# 4 @ 10.00 in		
Heel Reinforcing	=	# 4 @ 10.00 in		
Key Reinforcing	=	None Spec'd		
Footing Torsion, Tu	=		0.00	ft-lbs
Footing Allow. Torsion, φ Tn	=		0.00	ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 18 in, #7@ 18 in, #8@ 18 in, #9@ 18 in, #10@ 18 in

Heel: #4@ 11.11 in, #5@ 17.22 in, #6@ 18 in, #7@ 18 in, #8@ 18 in, #9@ 18 in, #10@ 18 in

Key: No key defined

Min footing T&S reinf Area 0.83 in2
 Min footing T&S reinf Area per foot 0.22 in2 /ft

<u>If one layer of horizontal bars:</u>	<u>If two layers of horizontal bars:</u>
#4@ 11.11 in	#4@ 22.22 in
#5@ 17.22 in	#5@ 34.44 in
#6@ 24.44 in	#6@ 48.89 in

Cantilevered Retaining Wall

Project Title: 4115 78th AVE SE

LIC# : KW-06015659, Build:20.25.03.24

MYERS ENGINEERING

(c) ENERCALC, LLC 1982-2025

DESCRIPTION: 8ft Stem at Basement w/ Slab

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	1,215.3	2.78	3,375.8	Soil Over HL (ab. water tbl)	549.7	3.50	1,923.9
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		3.50	1,923.9
Hydrostatic Force				Water Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =	150.0	2.94	440.6
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	137.5	1.25	171.9
				Surcharge Over Toe =			
				Stem Weight(s) =	800.0	2.83	2,266.7
				Earth @ Stem Transitions =			
Total	= 1,215.3	O.T.M. =	3,375.8	Footing Weight =	479.1	1.92	918.2
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		= 1.69		Total =	2,116.4 lbs	R.M. =	5,721.4
Vertical Loads used for Soil Pressure =		2,652.8 lbs		* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.			

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.056 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Cantilevered Retaining Wall

Project Title: 4115 78th AVE SE

LIC# : KW-06015659, Build:20.25.03.24

MYERS ENGINEERING

(c) ENERCALC, LLC 1982-2025

DESCRIPTION: 8ft Stem at Basement w/ Slab

Rebar Lap & Embedment Lengths Information

Stem Design Segment: 2nd

Stem Design Height: 2.50 ft above top of footing

Lap Splice length for #4 bar specified in this stem design segment (25.4.2.3a) = 18.72 in
Development length for #4 bar specified in this stem design segment = 14.40 in

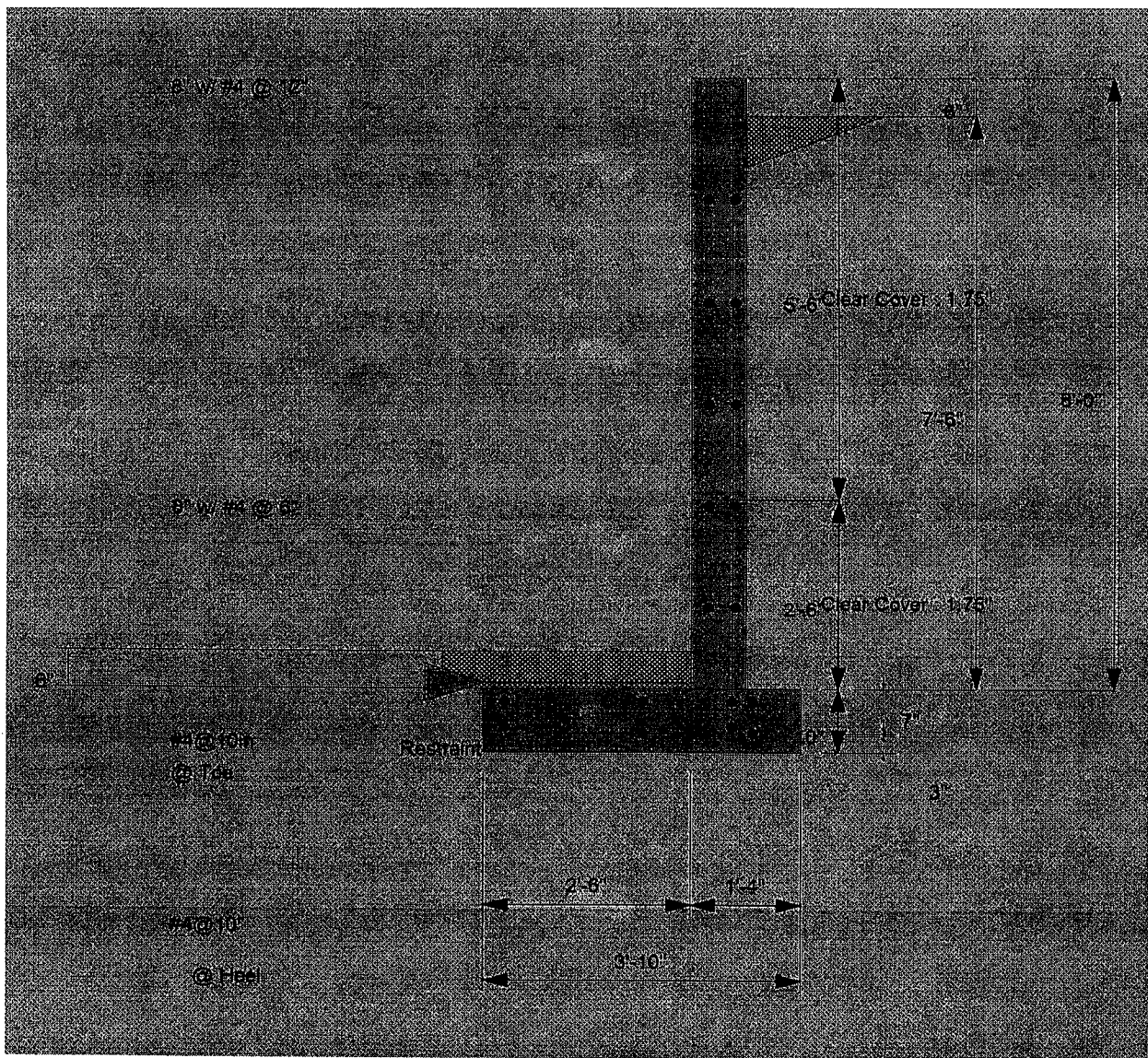
Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

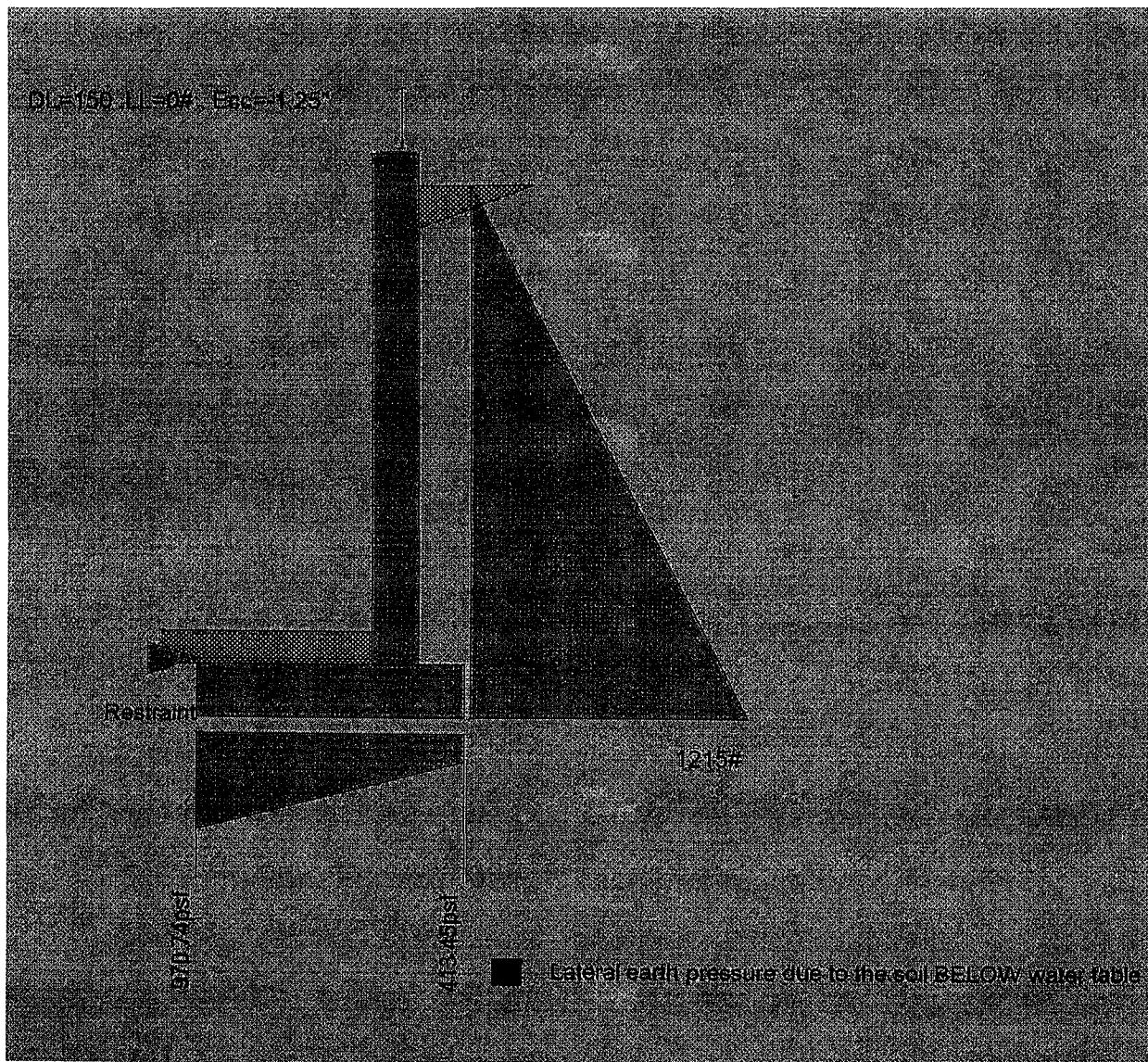
Lap Splice length for #4 bar specified in this stem design segment (25.4.2.3a) = 18.72 in
Development length for #4 bar specified in this stem design segment = 14.40 in

Hooked embedment length into footing for #4 bar specified in this stem design segment = 6.00 in
As Provided = 0.4000 in²/ft
As Required = 0.2044 in²/ft

DESCRIPTION: 8ft Stem at Basement w/ Slab



DESCRIPTION: 8ft Stem at Basement w/ Slab



DESCRIPTION: 8ft Stem at Basement w/ Slab & Seismic

Code Reference:

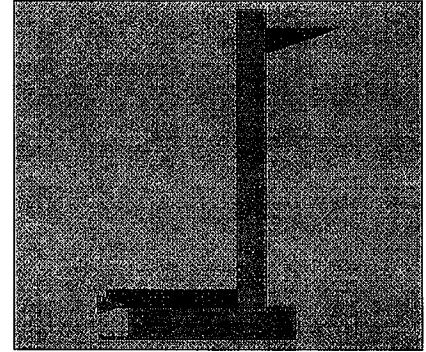
Calculations per IBC 2018, ACI 318-14, TMS 402-16

Criteria

Retained Height	=	7.50 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water table above bottom of footing	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,500.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0 psf
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	150.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	1.3 in

Earth Pressure Seismic Load

Method : Uniform		
Multiplier Used	=	8.000
(Multiplier used on soil density)		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Uniform Seismic Force	=	66.667
Total Seismic Force	=	555.556

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

DESCRIPTION: 8ft Stem at Basement w/ Slab & Seismic

Design Summary

Wall Stability Ratios

Overturning	=	1.15 Ratio < 1.5!
Slab Resists All Sliding !		
Global Stability	=	1.35
Total Bearing Load	=	2,653 lbs
...resultant ecc.	=	10.42 in
Eccentricity outside middle third		
Soil Pressure @ Toe	=	1,687 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,500 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	1,884 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	26.6 psi OK
Footing Shear @ Heel	=	53.9 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	1,604.2 lbs
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Vertical component of active lateral soil pressure IS considered in the calculation of soil bearing pressures.

Load Factors

Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

Stem Construction

Design Height Above Ftg

Wall Material Above "Ht"	=	Concrete
Design Method	=	SD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	12.00
Rebar Placed at	=	6 in

Design Data

fb/FB + fa/Fa	=	0.381	0.582
Total Force @ Section			
Service Level	lbs =		
Strength Level	lbs =	1,033.3	2,075.0
Moment....Actual			
Service Level	ft-# =		
Strength Level	ft-# =	1,981.3	5,793.8
Moment....Allowable	ft-# =	5,187.6	9,950.4
Shear.....Actual			
Service Level	psi =		
Strength Level	psi =	14.4	28.8
Shear....Allowable	psi =	75.0	75.0
Anet (Masonry)	in2 =		
Wall Weight	psf =	100.0	100.0
Rebar Depth 'd'	in =	6.00	6.00

Masonry Data

f _m	psi =
F _s	psi =
Solid Grouting	=
Modular Ratio 'n'	=
Equiv. Solid Thick.	=
Masonry Block Type	=
Masonry Design Method	= ASD

Concrete Data

f _c	psi =	2,500.0	2,500.0
F _y	psi =	60,000.0	60,000.0

DESCRIPTION: 8ft Stem at Basement w/ Slab & Seismic

Concrete Stem Rebar Area Details

2nd Stem	<u>Vertical Reinforcing</u>	<u>Horizontal Reinforcing</u>
As (based on applied moment) :	0.0775 in2/ft	
(4/3) * As :	0.1033 in2/ft	Min Stem T&S Reinf Area 1.056 in2
200bd/fy : 200(12)(6)/60000 :	0.24 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft
0.0018bh : 0.0018(12)(8) :	0.1728 in2/ft	Horizontal Reinforcing Options :
	=====	<u>One layer of :</u> <u>Two layers of :</u>
Required Area :	0.1728 in2/ft	#4@ 12.50 in #4@ 25.00 in
Provided Area :	0.2 in2/ft	#5@ 19.38 in #5@ 38.75 in
Maximum Area :	0.8128 in2/ft	#6@ 27.50 in #6@ 55.00 in

Bottom Stem	<u>Vertical Reinforcing</u>	<u>Horizontal Reinforcing</u>
As (based on applied moment) :	0.2266 in2/ft	
(4/3) * As :	0.3022 in2/ft	Min Stem T&S Reinf Area 0.480 in2
200bd/fy : 200(12)(6)/60000 :	0.24 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft
0.0018bh : 0.0018(12)(8) :	0.1728 in2/ft	Horizontal Reinforcing Options :
	=====	<u>One layer of :</u> <u>Two layers of :</u>
Required Area :	0.24 in2/ft	#4@ 12.50 in #4@ 25.00 in
Provided Area :	0.4 in2/ft	#5@ 19.38 in #5@ 38.75 in
Maximum Area :	0.8128 in2/ft	#6@ 27.50 in #6@ 55.00 in

Footing Data

Toe Width	=	2.50 ft
Heel Width	=	1.33
Total Footing Width	=	3.83
Footing Thickness	=	10.00 in

f _c =	2,500 psi	F _y =	60,000 psi
Footing Concrete Density	=	150.00 pcf	
Min. As %	=	0.0018	
Cover @ Top	7.00	@ Btm.=	3.00 in

Footing Design Results

		<u>Toe</u>	<u>Heel</u>	
Factored Pressure	=	1,884	0	psf
Mu' : Upward	=	4,328	0	ft-#
Mu' : Downward	=	675	825	ft-#
Mu: Design	=	3,653	825	ft-#
φ Mn	=	6,985	2,665	ft-#
Actual 1-Way Shear	=	26.60	53.93	psi
Allow 1-Way Shear	=	75.00	75.00	psi
Toe Reinforcing	=	# 4 @ 10.00 in		
Heel Reinforcing	=	# 4 @ 10.00 in		
Key Reinforcing	=	None Spec'd		
Footing Torsion, Tu	=		0.00	ft-lbs
Footing Allow. Torsion, φ Tn	=		0.00	ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 18 in, #7@ 18 in, #8@ 18 in, #9@ 18 in, #10@ 18 in

Heel: #4@ 18 in, #5@ 18 in, #6@ 18 in, #7@ 18 in, #8@ 18 in, #9@ 18 in, #10@ 18 in

Key: No key defined

Min footing T&S reinf Area	0.83 in2
Min footing T&S reinf Area per foot	0.22 in2 /ft

If one layer of horizontal bars:

- #4@ 11.11 in
- #5@ 17.22 in
- #6@ 24.44 in

If two layers of horizontal bars:

- #4@ 22.22 in
- #5@ 34.44 in
- #6@ 48.89 in

DESCRIPTION: 8ft Stem at Basement w/ Slab & Seismic

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	1,215.3	2.78	3,375.8	Soil Over HL (ab. water tbl)	549.7	3.50	1,923.9
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		3.50	1,923.9
Hydrostatic Force				Water Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =	150.0	2.94	440.6
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	137.5	1.25	171.9
Seismic Earth Load =	388.9	4.17	1,620.4	Surcharge Over Toe =			
				Stem Weight(s) =	800.0	2.83	2,266.7
				Earth @ Stem Transitions =			
Total	1,604.2	O.T.M. =	4,996.1	Footing Weight =	479.1	1.92	918.2
				Key Weight =			
Resisting/Overturning Ratio		=	1.15	Vert. Component =			
Vertical Loads used for Soil Pressure =		2,652.8 lbs		Total =	2,116.4 lbs	R.M. =	5,721.4

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

If seismic is included, the OTM and sliding ratios may be 1.1 per section 1807.2.3 of IBC.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.098 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

DESCRIPTION: 8ft Stem at Basement w/ Slab & Seismic

Rebar Lap & Embedment Lengths Information

Stem Design Segment: 2nd

Stem Design Height: 2.50 ft above top of footing

Lap Splice length for #4 bar specified in this stem design segment (25.4.2.3a) = 18.72 in
Development length for #4 bar specified in this stem design segment = 14.40 in

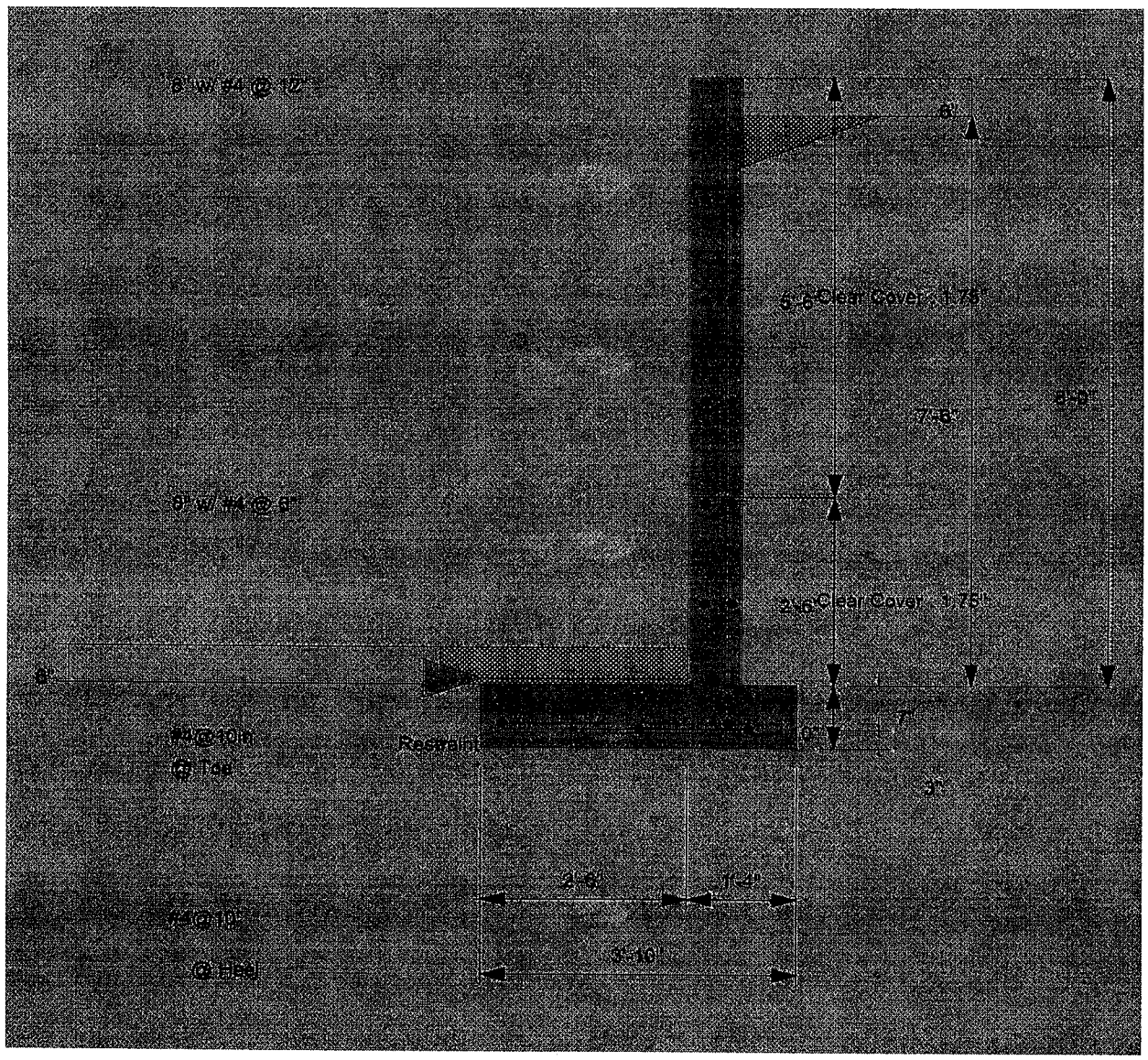
Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

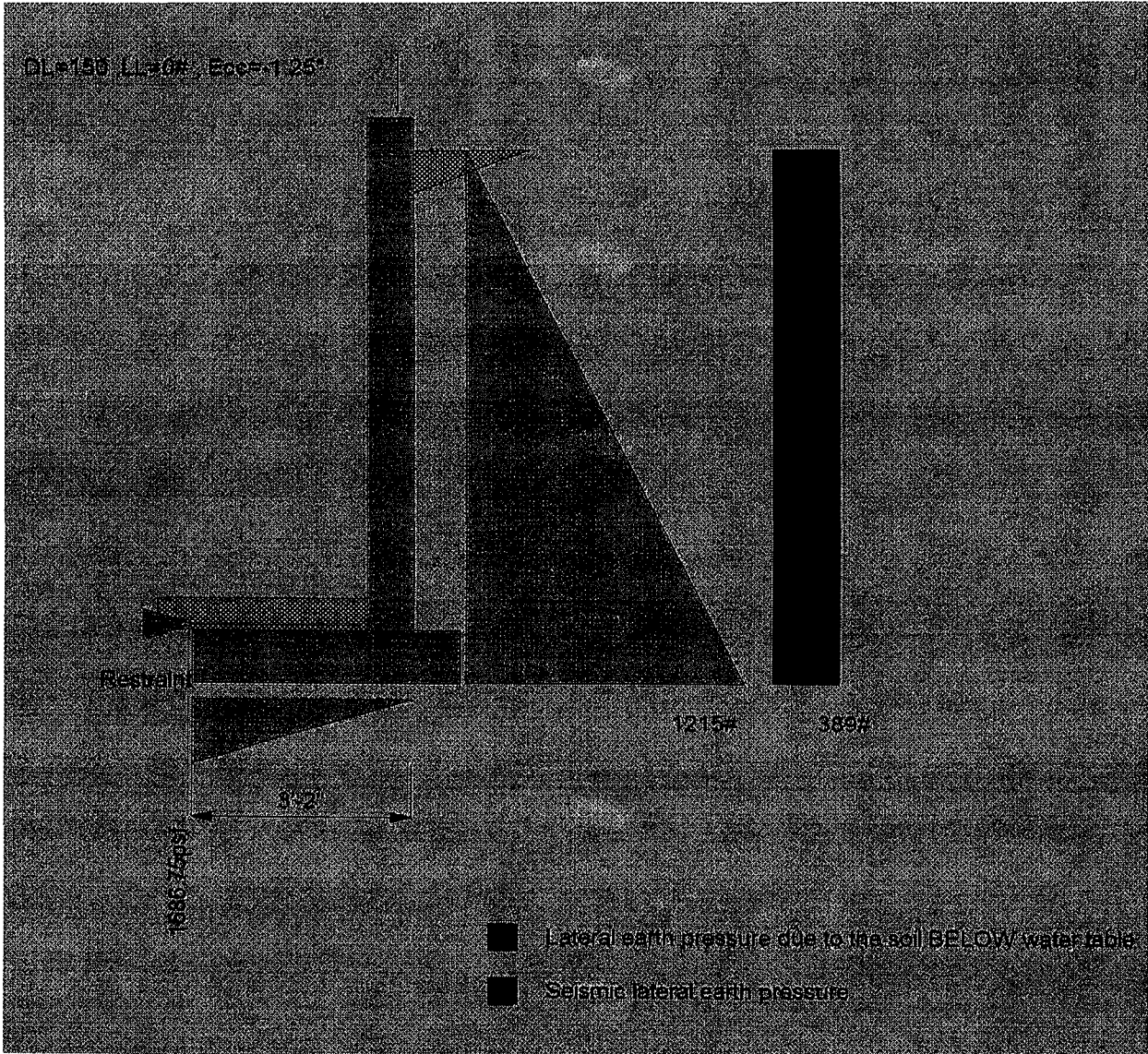
Lap Splice length for #4 bar specified in this stem design segment (25.4.2.3a) = 18.72 in
Development length for #4 bar specified in this stem design segment = 14.40 in

Hooked embedment length into footing for #4 bar specified in this stem design segment = 6.00 in
As Provided = 0.4000 in²/ft
As Required = 0.2400 in²/ft

DESCRIPTION: 8ft Stem at Basement w/ Slab & Seismic



DESCRIPTION: 8ft Stem at Basement w/ Slab & Seismic



DESCRIPTION: 6ft Stem at Basement w/ Slab

Code Reference:

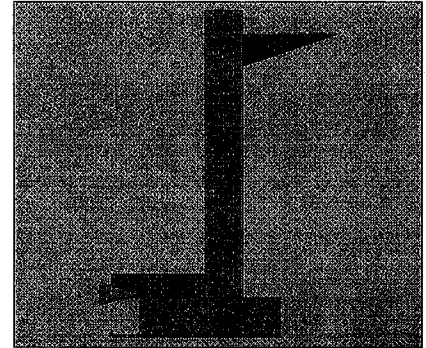
Calculations per IBC 2018, ACI 318-14, TMS 402-16

Criteria

Retained Height	=	5.50 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water table above bottom of footing	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,500.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0 psf
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	150.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	1.3 in

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

DESCRIPTION: 6ft Stem at Basement w/ Slab

Design Summary

Wall Stability Ratios

Overturning = 1.65 OK

Slab Resists All Sliding !

Global Stability = 1.76

Total Bearing Load = 1,840 lbs

...resultant ecc. = 3.68 in

Eccentricity within middle third

Soil Pressure @ Toe = 1,278 psf OK

Soil Pressure @ Heel = 194 psf OK

Allowable = 2,500 psf

Soil Pressure Less Than Allowable

ACI Factored @ Toe = 1,487 psf

ACI Factored @ Heel = 226 psf

Footing Shear @ Toe = 8.8 psi OK

Footing Shear @ Heel = 27.2 psi OK

Allowable = 75.0 psi

Sliding Calcs

Lateral Sliding Force = 701.9 lbs

Vertical component of active lateral soil pressure IS considered in the calculation of soil bearing pressures.

Load Factors

Building Code

Dead Load 1.200

Live Load 1.600

Earth, H 1.600

Wind, W 1.600

Seismic, E 1.000

Stem Construction

Design Height Above Ftg

Wall Material Above "Ht" = Concrete

Design Method = SD

Thickness = 8.00

Rebar Size = # 4

Rebar Spacing = 10.00

Rebar Placed at = 6 in

Design Data

fb/FB + fa/Fa = 0.037

Total Force @ Section

Service Level lbs =

Strength Level lbs = 252.0 847.0

Moment....Actual

Service Level ft-# =

Strength Level ft-# = 233.3 1,534.1

Moment....Allowable

ft-# = 6,174.1 6,174.1

Shear.....Actual

Service Level psi =

Strength Level psi = 3.5 11.8

Shear.....Allowable

psi = 75.0 75.0

Anet (Masonry)

in2 =

Wall Weight

psf = 100.0 100.0

Rebar Depth 'd'

in = 6.00 6.00

Masonry Data

f'm psi =

Fs psi =

Solid Grouting =

Modular Ratio 'n' =

Equiv. Solid Thick. =

Masonry Block Type =

Masonry Design Method = ASD

Concrete Data

f'c psi = 2,500.0 2,500.0

Fy psi = 60,000.0 60,000.0

DESCRIPTION: 6ft Stem at Basement w/ Slab

Concrete Stem Rebar Area Details

2nd Stem	<u>Vertical Reinforcing</u>	<u>Horizontal Reinforcing</u>
As (based on applied moment) :	0.0091 in2/ft	
(4/3) * As :	0.0122 in2/ft	Min Stem T&S Reinf Area 0.672 in2
200bd/fy : 200(12)(6)/60000 :	0.24 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft
0.0018bh : 0.0018(12)(8) :	0.1728 in2/ft	Horizontal Reinforcing Options :
	=====	<u>One layer of :</u> <u>Two layers of :</u>
Required Area :	0.1728 in2/ft	#4@ 12.50 in #4@ 25.00 in
Provided Area :	0.24 in2/ft	#5@ 19.38 in #5@ 38.75 in
Maximum Area :	0.8128 in2/ft	#6@ 27.50 in #6@ 55.00 in

Bottom Stem	<u>Vertical Reinforcing</u>	<u>Horizontal Reinforcing</u>
As (based on applied moment) :	0.06 in2/ft	
(4/3) * As :	0.08 in2/ft	Min Stem T&S Reinf Area 0.480 in2
200bd/fy : 200(12)(6)/60000 :	0.24 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft
0.0018bh : 0.0018(12)(8) :	0.1728 in2/ft	Horizontal Reinforcing Options :
	=====	<u>One layer of :</u> <u>Two layers of :</u>
Required Area :	0.1728 in2/ft	#4@ 12.50 in #4@ 25.00 in
Provided Area :	0.24 in2/ft	#5@ 19.38 in #5@ 38.75 in
Maximum Area :	0.8128 in2/ft	#6@ 27.50 in #6@ 55.00 in

Footing Data

Toe Width	=	1.17 ft
Heel Width	=	1.33
Total Footing Width	=	2.50
Footing Thickness	=	10.00 in
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	7.00	@ Btm.= 3.00 in

Footing Design Results

		<u>Toe</u>	<u>Heel</u>	
Factored Pressure	=	1,487	226	psf
Mu' : Upward	=	879	75	ft-#
Mu' : Downward	=	147	525	ft-#
Mu: Design	=	732	450	ft-#
φ Mn	=	6,985	2,665	ft-#
Actual 1-Way Shear	=	8.75	27.23	psi
Allow 1-Way Shear	=	75.00	75.00	psi
Toe Reinforcing	=	# 4 @ 10.00 in		
Heel Reinforcing	=	# 4 @ 10.00 in		
Key Reinforcing	=	None Spec'd		
Footing Torsion, Tu	=		0.00	ft-lbs
Footing Allow. Torsion, φ Tn	=		0.00	ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 18 in, #7@ 18 in, #8@ 18 in, #9@ 18 in, #10@ 18 in

Heel: #4@ 11.11 in, #5@ 17.22 in, #6@ 18 in, #7@ 18 in, #8@ 18 in, #9@ 18 in, #10@ 18 in

Key: No key defined

Min footing T&S reinf Area 0.54 in2
 Min footing T&S reinf Area per foot 0.22 in2 /ft

If one layer of horizontal bars: If two layers of horizontal bars:

#4@ 11.11 in	#4@ 22.22 in
#5@ 17.22 in	#5@ 34.44 in
#6@ 24.44 in	#6@ 48.89 in

DESCRIPTION: 6ft Stem at Basement w/ Slab

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	701.9	2.11	1,481.9	Soil Over HL (ab. water tbl)	403.1	2.17	873.5
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		2.17	873.5
Hydrostatic Force				Water Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =	150.0	1.60	240.7
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	64.2	0.58	37.5
				Surcharge Over Toe =			
				Stem Weight(s) =	600.0	1.50	900.2
				Earth @ Stem Transitions =			
Total	= 701.9	O.T.M. =	1,481.9	Footing Weight =	312.5	1.25	390.6
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		= 1.65		Total =	1,529.8 lbs	R.M. =	2,442.5
Vertical Loads used for Soil Pressure =		1,839.7 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.085 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

DESCRIPTION: 6ft Stem at Basement w/ Slab

Rebar Lap & Embedment Lengths Information

Stem Design Segment: 2nd

Stem Design Height: 2.50 ft above top of footing

Lap Splice length for #4 bar specified in this stem design segment (25.4.2.3a) = 18.72 in
Development length for #4 bar specified in this stem design segment = 14.40 in

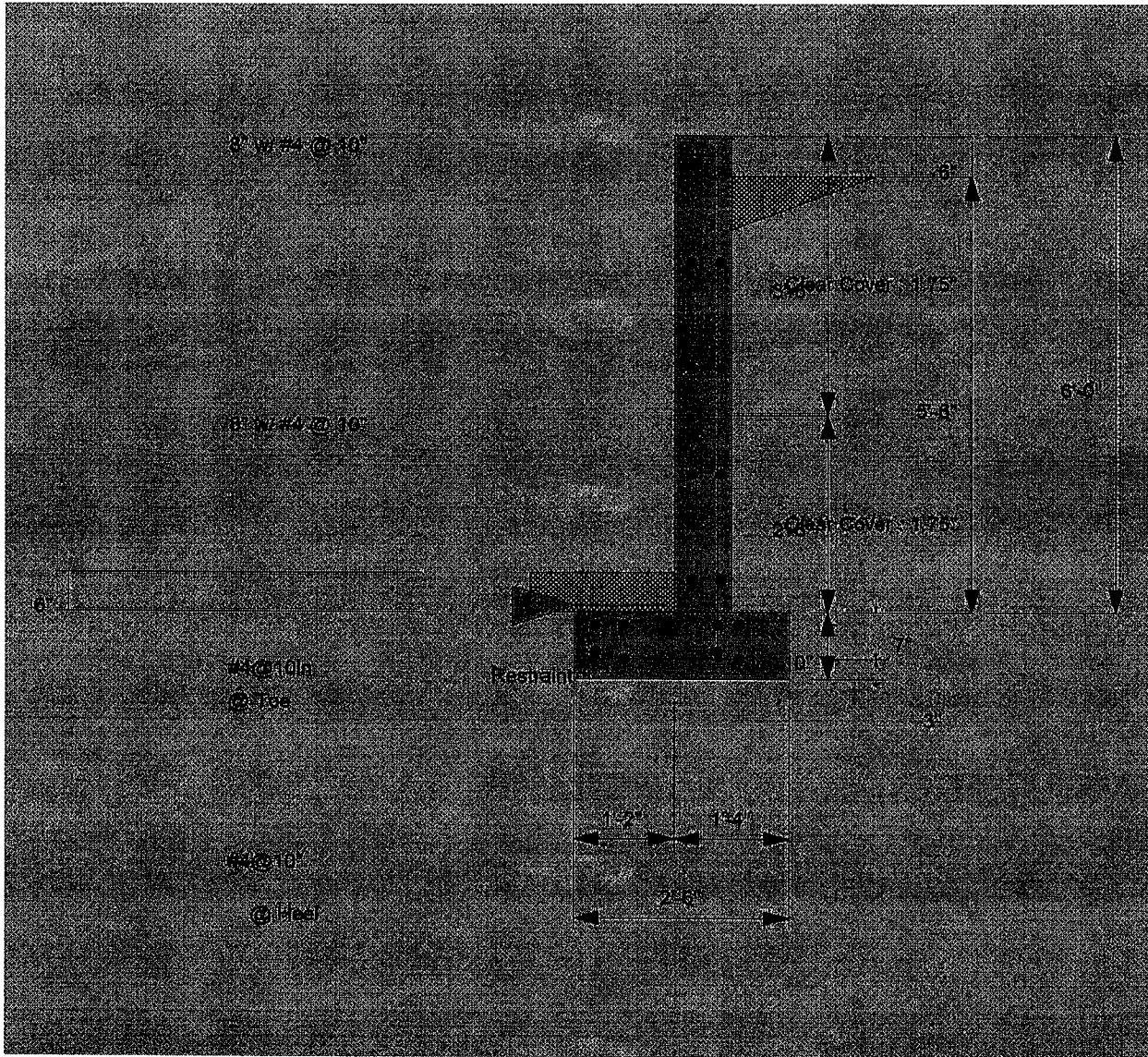
Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

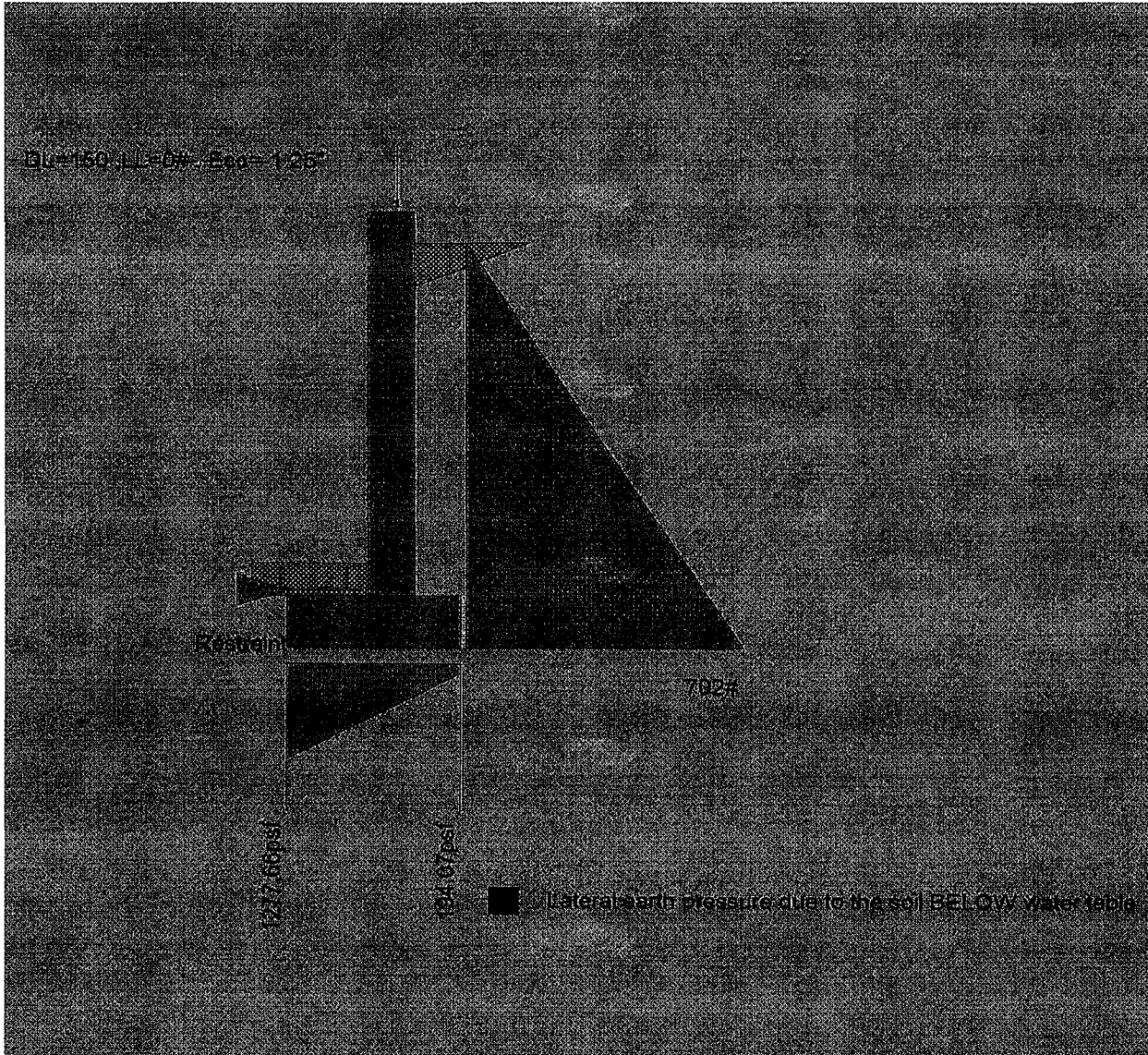
Lap Splice length for #4 bar specified in this stem design segment (25.4.2.3a) = 18.72 in
Development length for #4 bar specified in this stem design segment = 14.40 in

Hooked embedment length into footing for #4 bar specified in this stem design segment = 6.05 in
As Provided = 0.2400 in²/ft
As Required = 0.1728 in²/ft

DESCRIPTION: 6ft Stem at Basement w/ Slab



DESCRIPTION: 6ft Stem at Basement w/ Slab



DESCRIPTION: 6ft Stem at Basement w/ Slab & Seismic

Code Reference:

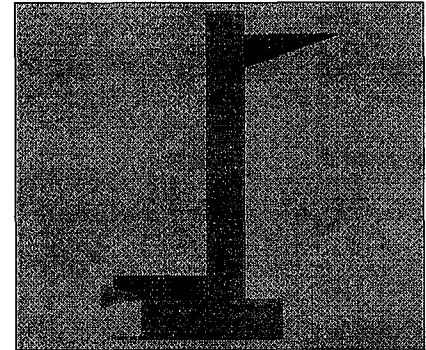
Calculations per IBC 2018, ACI 318-14, TMS 402-16

Criteria

Retained Height	=	5.50 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water table above bottom of footing	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,500.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.0 pcf
Soil Density, Toe	=	110.0 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0 psf
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	150.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	1.3 in

Earth Pressure Seismic Load

Method : Uniform		
Multiplier Used	=	8.000
(Multiplier used on soil density)		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Uniform Seismic Force	=	50.667
Total Seismic Force	=	320.889

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

DESCRIPTION: 6ft Stem at Basement w/ Slab & Seismic

Design Summary		Stem Construction		2nd	Bottom
Wall Stability Ratios		Design Height Above Ftg	ft =	Stem OK 2.50	Stem OK 0.00
Overturning	= 1.11 Ratio < 1.5!	Wall Material Above "Ht"	=	Concrete	Concrete
Slab Resists All Sliding !		Design Method	=	SD	SD
Global Stability	= 1.76	Thickness	=	8.00	8.00
Total Bearing Load	= 1,840 lbs	Rebar Size	=	# 4	# 4
...resultant ecc.	= 8.32 in	Rebar Spacing	=	10.00	10.00
Eccentricity outside middle third		Rebar Placed at	=	6 in	6 in
Soil Pressure @ Toe	= 2,204 psf OK	Design Data			
Soil Pressure @ Heel	= 0 psf OK	fb/FB + fa/Fa	=	0.074	0.372
Allowable	= 2,500 psf	Total Force @ Section			
Soil Pressure Less Than Allowable		Service Level	lbs =		
ACI Factored @ Toe	= 2,565 psf	Strength Level	lbs =	404.0	1,125.7
ACI Factored @ Heel	= 0 psf	Moment....Actual			
Footing Shear @ Toe	= 14.8 psi OK	Service Level	ft-# =		
Footing Shear @ Heel	= 36.0 psi OK	Strength Level	ft-# =	461.3	2,300.4
Allowable	= 75.0 psi	Moment....Allowable	ft-# =	6,174.1	6,174.1
Sliding Calcs		Shear.....Actual			
Lateral Sliding Force	= 926.6 lbs	Service Level	psi =		
		Strength Level	psi =	5.6	15.6
		Shear....Allowable	psi =	75.0	75.0
		Anet (Masonry)	in2 =		
		Wall Weight	psf =	100.0	100.0
		Rebar Depth 'd'	in =	6.00	6.00
Vertical component of active lateral soil pressure IS considered in the calculation of soil bearing pressures.		Masonry Data			
		f _m	psi =		
		F _s	psi =		
		Solid Grouting	=		
		Modular Ratio 'n'	=		
		Equiv. Solid Thick.	=		
		Masonry Block Type	=		
		Masonry Design Method	=	ASD	
Load Factors		Concrete Data			
Building Code		f _c	psi =	2,500.0	2,500.0
Dead Load	1.200	F _y	psi =	60,000.0	60,000.0
Live Load	1.600				
Earth, H	1.600				
Wind, W	1.600				
Seismic, E	1.000				

DESCRIPTION: 6ft Stem at Basement w/ Slab & Seismic

Concrete Stem Rebar Area Details

2nd Stem	<u>Vertical Reinforcing</u>	<u>Horizontal Reinforcing</u>
As (based on applied moment) :	0.018 in2/ft	
(4/3) * As :	0.0241 in2/ft	Min Stem T&S Reinf Area 0.672 in2
200bd/fy : 200(12)(6)/60000 :	0.24 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft
0.0018bh : 0.0018(12)(8) :	0.1728 in2/ft	Horizontal Reinforcing Options :
	=====	<u>One layer of :</u> <u>Two layers of :</u>
Required Area :	0.1728 in2/ft	#4@ 12.50 in #4@ 25.00 in
Provided Area :	0.24 in2/ft	#5@ 19.38 in #5@ 38.75 in
Maximum Area :	0.8128 in2/ft	#6@ 27.50 in #6@ 55.00 in

Bottom Stem	<u>Vertical Reinforcing</u>	<u>Horizontal Reinforcing</u>
As (based on applied moment) :	0.09 in2/ft	
(4/3) * As :	0.12 in2/ft	Min Stem T&S Reinf Area 0.480 in2
200bd/fy : 200(12)(6)/60000 :	0.24 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft
0.0018bh : 0.0018(12)(8) :	0.1728 in2/ft	Horizontal Reinforcing Options :
	=====	<u>One layer of :</u> <u>Two layers of :</u>
Required Area :	0.1728 in2/ft	#4@ 12.50 in #4@ 25.00 in
Provided Area :	0.24 in2/ft	#5@ 19.38 in #5@ 38.75 in
Maximum Area :	0.8128 in2/ft	#6@ 27.50 in #6@ 55.00 in

Footing Data

Toe Width	=	1.17 ft
Heel Width	=	1.33
Total Footing Width	=	2.50
Footing Thickness	=	10.00 in
f _c =	2,500 psi	F _y = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	7.00	@ Btm.= 3.00 in

Footing Design Results

		<u>Toe</u>	<u>Heel</u>	
Factored Pressure	=	2,565	0	psf
Mu' : Upward	=	1,340	0	ft-#
Mu' : Downward	=	147	525	ft-#
Mu: Design	=	1,193	525	ft-#
φ Mn	=	6,985	2,665	ft-#
Actual 1-Way Shear	=	14.81	35.98	psi
Allow 1-Way Shear	=	75.00	75.00	psi
Toe Reinforcing	=	# 4 @ 10.00 in		
Heel Reinforcing	=	# 4 @ 10.00 in		
Key Reinforcing	=	None Spec'd		
Footing Torsion, Tu	=		0.00 ft-lbs	
Footing Allow. Torsion, φ Tn	=		0.00 ft-lbs	

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 18 in, #7@ 18 in, #8@ 18 in, #9@ 18 in, #10@ 18 in

Heel: #4@ 11.11 in, #5@ 17.22 in, #6@ 18 in, #7@ 18 in, #8@ 18 in, #9@ 18 in, #10@ 18 in

Key: No key defined

Min footing T&S reinf Area 0.54 in2
 Min footing T&S reinf Area per foot 0.22 in2 /ft

<u>If one layer of horizontal bars:</u>	<u>If two layers of horizontal bars:</u>
#4@ 11.11 in	#4@ 22.22 in
#5@ 17.22 in	#5@ 34.44 in
#6@ 24.44 in	#6@ 48.89 in

DESCRIPTION: 6ft Stem at Basement w/ Slab & Seismic

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	701.9	2.11	1,481.9	Soil Over HL (ab. water tbl)	403.1	2.17	873.5
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		2.17	873.5
Hydrostatic Force				Water Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =	150.0	1.60	240.7
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	64.2	0.58	37.5
Seismic Earth Load =	224.6	3.17	711.3	Surcharge Over Toe =			
				Stem Weight(s) =	600.0	1.50	900.2
				Earth @ Stem Transitions =			
Total	926.6	O.T.M. =	2,193.2	Footing Weight =	312.5	1.25	390.6
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		=	1.11	Total =	1,529.8 lbs	R.M. =	2,442.5
Vertical Loads used for Soil Pressure =			1,839.7 lbs				

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

If seismic is included, the OTM and sliding ratios may be 1.1 per section 1807.2.3 of IBC.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.147 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

DESCRIPTION: 6ft Stem at Basement w/ Slab & Seismic

Rebar Lap & Embedment Lengths Information

Stem Design Segment: 2nd

Stem Design Height: 2.50 ft above top of footing

Lap Splice length for #4 bar specified in this stem design segment (25.4.2.3a) = 18.72 in
Development length for #4 bar specified in this stem design segment = 14.40 in

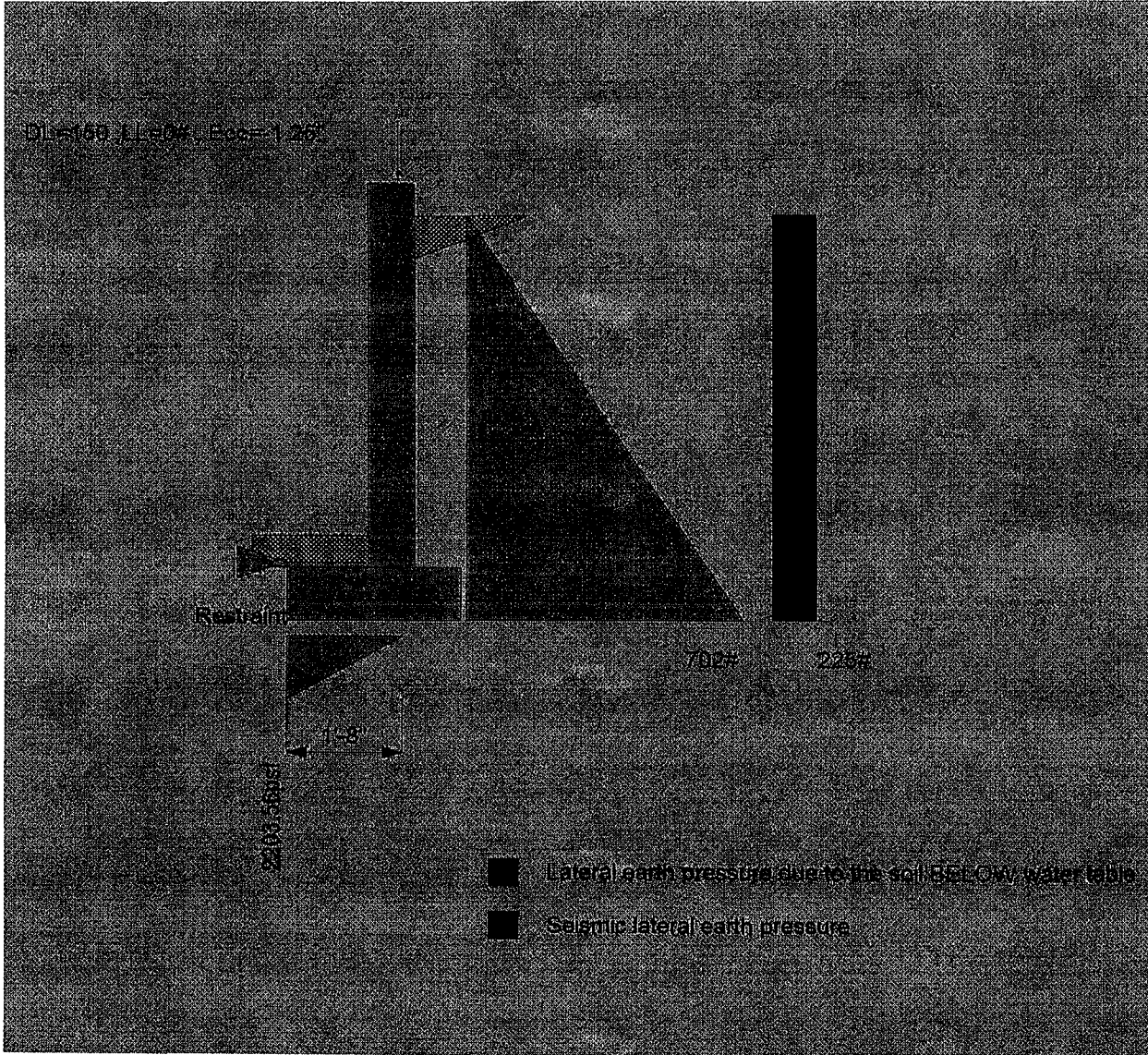
Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #4 bar specified in this stem design segment (25.4.2.3a) = 18.72 in
Development length for #4 bar specified in this stem design segment = 14.40 in

Hooked embedment length into footing for #4 bar specified in this stem design segment = 6.05 in
As Provided = 0.2400 in²/ft
As Required = 0.1728 in²/ft

DESCRIPTION: 6ft Stem at Basement w/ Slab & Seismic



DESCRIPTION: 4ft Stem at Basement w/ Slab

Code Reference:

Calculations per IBC 2018, ACI 318-14, TMS 402-16

Criteria

Retained Height	=	3.50 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water table above bottom of footing	=	0.0 ft

Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
Used for Sliding & Overturning		

Axial Load Applied to Stem

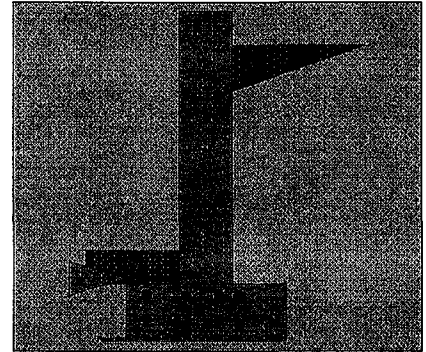
Axial Dead Load	=	150.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	1.3 in

Soil Data

Allow Soil Bearing	=	2,500.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)



Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

DESCRIPTION: 4ft Stem at Basement w/ Slab

Design Summary

Wall Stability Ratios

Overturning	=	2.64	OK
Slab Resists All Sliding !			
Global Stability	=	2.53	
Total Bearing Load	=	1,238	lbs
...resultant ecc.	=	1.62	in
Eccentricity within middle third			
Soil Pressure @ Toe	=	870	psf OK
Soil Pressure @ Heel	=	368	psf OK
Allowable	=	2,500	psf
Soil Pressure Less Than Allowable			
ACI Factored @ Toe	=	1,076	psf
ACI Factored @ Heel	=	455	psf
Footing Shear @ Toe	=	1.3	psi OK
Footing Shear @ Heel	=	8.9	psi OK
Allowable	=	75.0	psi

Sliding Calcs

Lateral Sliding Force	=	328.6	lbs
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Vertical component of active lateral soil pressure IS considered in the calculation of soil bearing pressures.

Load Factors

Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

Stem Construction

Design Height Above Ftg

ft =	Stem OK	0.00
Wall Material Above "H"	=	Concrete
Design Method	=	SD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	10.00
Rebar Placed at	=	6 in

Design Data

fb/FB + fa/Fa	=	0.061
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	343.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	381.4

Moment....Allowable	=	6,174.1
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Shear.....Actual

Service Level	psi =	
Strength Level	psi =	4.8

Shear.....Allowable	psi =	75.0
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Anet (Masonry)	in2 =	
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Wall Weight	psf =	100.0
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Rebar Depth 'd'	in =	6.00
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Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

DESCRIPTION: 4ft Stem at Basement w/ Slab

Concrete Stem Rebar Area Details

Bottom Stem	<u>Vertical Reinforcing</u>	<u>Horizontal Reinforcing</u>	
As (based on applied moment) :	0.0149 in2/ft		
(4/3) * As :	0.0199 in2/ft	Min Stem T&S Reinf Area 0.768 in2	
200bd/fy : 200(12)(6)/60000 :	0.24 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in2/ft	Horizontal Reinforcing Options :	
	=====	<u>One layer of :</u> <u>Two layers of :</u>	
Required Area :	0.1728 in2/ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.24 in2/ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8128 in2/ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	0.67 ft
Heel Width	=	1.33
Total Footing Width	=	2.00
Footing Thickness	=	10.00 in

fc =	2,500 psi	Fy =	60,000 psi
Footing Concrete Density	=	150.00 pcf	
Min. As %	=	0.0018	
Cover @ Top	7.00	@ Btm.=	3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>	
Factored Pressure	= 1,076	455	psf
Mu' : Upward	= 224	116	ft-#
Mu' : Downward	= 48	291	ft-#
Mu: Design	= 176	174	ft-#
φ Mn	= 6,985	2,665	ft-#
Actual 1-Way Shear	= 1.31	8.93	psi
Allow 1-Way Shear	= 75.00	75.00	psi
Toe Reinforcing	= # 4 @ 10.00 in		
Heel Reinforcing	= # 4 @ 10.00 in		
Key Reinforcing	= None Spec'd		
Footing Torsion, Tu	=	0.00	ft-lbs
Footing Allow. Torsion, φ Tn	=	0.00	ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 18 in, #7@ 18 in, #8@ 18 in, #9@ 18 in, #10@ 18 in

Heel: #4@ 11.11 in, #5@ 17.22 in, #6@ 18 in, #7@ 18 in, #8@ 18 in, #9@ 18 in, #10@ 18 in

Key: No key defined

Min footing T&S reinf Area	0.43 in2
Min footing T&S reinf Area per foot	0.22 in2 /ft

If one layer of horizontal bars:

- #4@ 11.11 in
- #5@ 17.22 in
- #6@ 24.44 in

If two layers of horizontal bars:

- #4@ 22.22 in
- #5@ 34.44 in
- #6@ 48.89 in

DESCRIPTION: 4ft Stem at Basement w/ Slab

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	328.6	1.44	474.7	Soil Over HL (ab. water tbl)	256.5	1.67	427.5
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		1.67	427.5
Hydrostatic Force				Water Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =	150.0	1.10	165.6
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	36.7	0.33	12.2
				Surcharge Over Toe =			
				Stem Weight(s) =	400.0	1.00	400.0
				Earth @ Stem Transitions =			
Total	328.6	O.T.M. =	474.7	Footing Weight =	250.0	1.00	249.9
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		= 2.64		Total =	1,093.2 lbs	R.M. =	1,255.3
Vertical Loads used for Soil Pressure =		1,238.2 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only) 0.048 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

DESCRIPTION: 4ft Stem at Basement w/ Slab

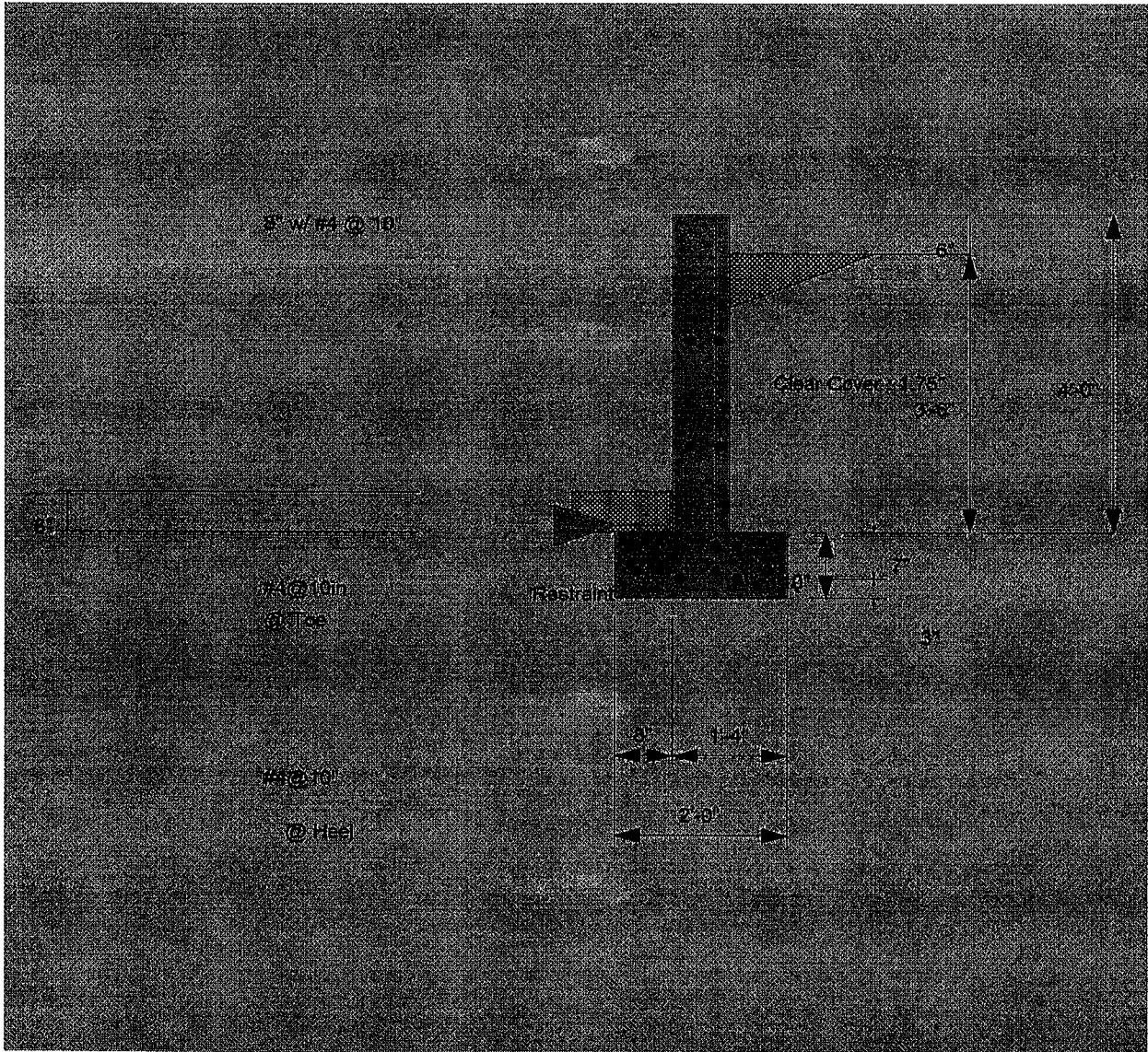
Rebar Lap & Embedment Lengths Information

Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #4 bar specified in this stem design segment (25.4.2.3a) =	18.72 in
Development length for #4 bar specified in this stem design segment =	14.40 in
Hooked embedment length into footing for #4 bar specified in this stem design segment =	6.05 in
As Provided =	0.2400 in ² /ft
As Required =	0.1728 in ² /ft

DESCRIPTION: 4ft Stem at Basement w/ Slab



DESCRIPTION: 4ft Stem at Basement w/ Slab

